

C-25	S-300
Km	Ks
19.00	3.95
20.00	3.96
21.00	3.96
22.00	3.97
23.00	3.98
24.00	3.98
25.00	3.99
26.00	4.00
27.00	4.02
28.00	4.03
29.00	4.04
30.00	4.05
31.00	4.06
32.00	4.07
33.00	4.08
34.00	4.09
35.00	4.10
36.00	4.11
37.00	4.12
38.00	4.15
39.00	4.17
40.00	4.18
41.00	4.20
42.00	4.21
43.00	4.24
44.00	4.26
45.00	4.29
46.00	4.31
47.00	4.34
48.00	4.36
49.00	4.39
50.00	4.41
51.00	4.46
52.00	4.49
53.00	4.51
54.00	4.56
55.00	4.62
56.00	4.65
57.00	4.67
58.00	4.73

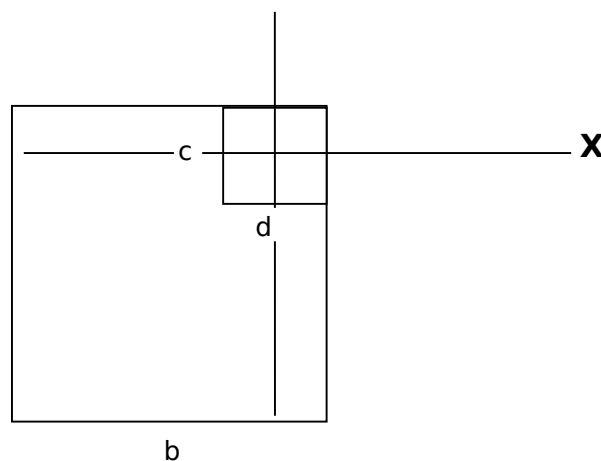
ISOLATED FOOTING DESIGN

I. MAXIMUM DESIGN LOAD

Ps=	278
Soil Wt. & own Wt. Of footing	2106.00
Pmax=	2384.00 KN
Mx=	1668 KNm
My=	1668 KNm
sall=	200 KN/m²

Column dimensions	c=	0.40 m
	d=	0.40 m

a



$$s = \frac{P}{A(1 + 6xe_a/a + 6e_b/b)}$$

Where s= the allowable soil pressure

$$A = a \times b \quad \text{ASSUME } a/b = 1.00$$

$$a = \text{length of footing} \quad 6.00 \quad 6.00$$

$$b = \text{width of footing} \quad 6.00 \quad 6.00$$

P= load sustained by the footing

$$e_a = 0.700$$

$$e_b = 0.700$$

$$s_1 = 158.89$$

$$s_2 = 66.22$$

$$s_3 = 66.22$$

$$s_4 = -26.44$$

$$s_{avg} = 66.22$$

DMT: Use the "tools-Goal seek" command and feed the allowable soil pressure to obtain this result. The value in "E19" should be used as a variable to change in order to achieve this result.

Structural design

use concrete strength 25 Mpa

$$F_{cd} = 11.167$$

$$F_{ctd} = 1.167$$

$$r_{min} = 0.00200$$

Steel S-300

$$V_{up} = 641.67$$

$$V_{ud} = 385.00$$

A) Punching Shear

$$d = 1.409 \text{ m}$$

taking concrete cover to be = 50 mm

$$D = 1459.376 \text{ mm}$$

$$\sim D = 1459.000 \text{ mm}$$

$$\text{hence } d = 1409.000 \text{ mm}$$

B) Wide beam shear

$$d_a = 817.98 \text{ mm}$$

$$d_b = 817.98 \text{ mm}$$

$$d = 1409.000 \text{ mm}$$

c) Bending moment

$$M_{a-a} = 259.59$$

$$M_{b-b} = 259.59$$

d) Reinforcement

$$A_{smin} = 2818$$

$$K_{da-a} = 0.0874513 \quad 11.43494$$

$$K_{db-b} = 0.0874513 \quad 11.43494$$

$$K_{sa-a} = \#N/A$$

$$K_{sb-b} = \#N/A$$

$$3.97$$

$$A_{sa-a} =$$

$$A_{sb-b} =$$

$$\#N/A \text{ mm}^2, \Phi 16c/c$$

$$\#N/A \text{ mm}^2, \Phi 16c/c$$

$$\#N/A$$

$$\#N/A$$

$$16 \quad 201$$

mm
mm

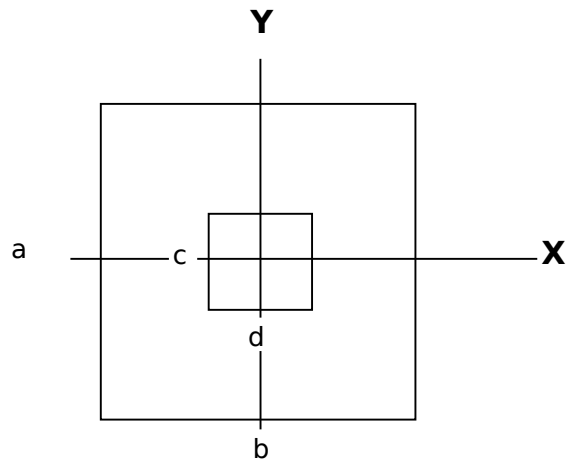
ISOLATED FOOTING DESIGN

I. MAXIMUM DESIGN LOAD

Ps=	1397
Soil Wt. & own Wt. Of footing	577.31
Pmax=	1974.31 KN
Mx=	0.5 KNm
My=	0.13 KNm

sall= 200 KN/m²

Column dimensions	c=	0.40 m
	d=	0.40 m



S= $P/A(1+6xe_a/a+6e_b/b)$

Where s= the allowable soil pressure

A= axb **ASSUME a/b=** 1.00

a= length of footing 3.14 3.14

b= width of footing 3.14 3.14

P= load sustained by the footing

ea= 0.000

eb= 0.000

s1= 200.00

s2= 200.31

s3= 199.81

s4= 199.76

savg= 199.97

DMT: Use the "tools-Goal seek" command and feed the allowable soil pressure to obtain this result. The value in "E19" should be used as a variable to change in order to achieve this result.

Structural design

use concrete strength 25 Mpa

Fcd= 11.167

Fctd= 1.167

r_{min}= 0.00200

Steel S-300

Vup= 641.67

Vud= 385.00

A) Punching Shear

d= 0.651 m

taking concrete cover to be = 50 mm

D= 700.682 mm

~D= 701.000 mm

hence d= 651.000 mm

B) Wide beam shear

da= 468.87 mm

db= 468.87 mm

d= 651.000 mm

c) Bending moment

Ma-a= 188.05

Mb-b= 188.05

D) Reinforcement

Asmin= 1302

Kda-a= 0.0474723 21.06491 Ksa-a= 3.96

Kdb-b= 0.0474723 21.06491 Ksb-b= 3.96

3.97

Asa-a= 1302 mm², **Φ14c/c** 14 154

Asb-b= 1302 mm², **Φ14c/c** 118

Asb-b= 1302 mm², **Φ14c/c** 118

mm
mm

8.462238

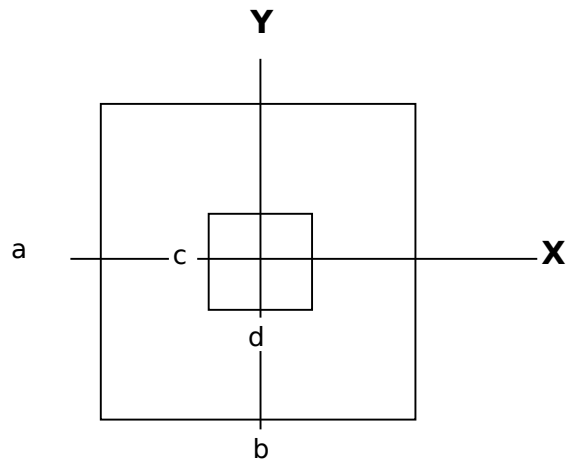
ISOLATED FOOTING DESIGN

I. MAXIMUM DESIGN LOAD

Ps=	855
Soil Wt. & own Wt. Of footing	357.07
Pmax=	1212.07 KN
Mx=	2.21 KNm
My=	1.61 KNm

sall= 200 KN/m²

Column dimensions	c=	0.40 m
	d=	0.40 m



s= $P/A(1+6xe_a/a+6e_b/b)$

Where s= the allowable soil pressure

A= axb **ASSUME a/b=** 1.00

a= length of footing 2.47 2.47

b= width of footing 2.47 2.47

P= load sustained by the footing

ea= 0.002

eb= 0.001

s1= 200.00

s2= 198.91

s3= 198.24

s4= 196.96

savg= 198.53

DMT: Use the "tools-Goal seek" command and feed the allowable soil pressure to obtain this result. The value in "E19" should be used as a variable to change in order to achieve this result.

Structural design

use concrete strength 25 Mpa

Fcd= 11.167

Fctd= 1.167

r_{min}= 0.00200

Steel S-300

Vup= 641.67

Vud= 385.00

A) Punching Shear

d= 0.475 m

taking concrete cover to be = 50 mm

D= 525.381 mm

~D= 525.000 mm

hence d= 475.000 mm

B) Wide beam shear

da= 354.05 mm

db= 354.05 mm

d= 475.000 mm

c) Bending moment

Ma-a= 106.46

Mb-b= 106.46

D) Reinforcement

Asmin= 950

Kda-a= 0.0460372 21.72158

Kdb-b= 0.0460372 21.72158

Ksa-a= 3.96

Ksb-b= 3.96

3.97

Asa-a= 950 mm², **Φ14c/c** 162

Asb-b= 950 mm², **Φ14c/c** 162

14 154

mm
mm

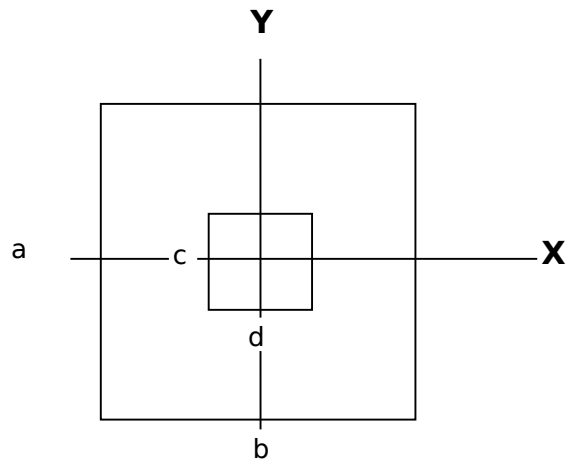
ISOLATED FOOTING DESIGN

I. MAXIMUM DESIGN LOAD

Ps=	681
Soil Wt. & own Wt. Of footing	288.56
Pmax=	969.56 KN
Mx=	0.7 KNm
My=	5.9 KNm

sall= 200 KN/m²

Column dimensions	c=	0.40 m
	d=	0.40 m



S= $P/A(1+6xe_a/a+6e_b/b)$

Where s= the allowable soil pressure

A= axb **ASSUME a/b=** 1.00

a= length of footing 2.22 2.22

b= width of footing 2.22 2.22

P= load sustained by the footing

ea= 0.001

eb= 0.006

s1= 200.00

s2= 193.88

s3= 199.23

s4= 192.78

savg= 196.47

DMT: Use the "tools-Goal seek" command and feed the allowable soil pressure to obtain this result. The value in "E19" should be used as a variable to change in order to achieve this result.

Structural design

use concrete strength 25 Mpa

Fcd= 11.167

Fctd= 1.167

r_{min}= 0.00200

Steel S-300

Vup= 641.67

Vud= 385.00

A) Punching Shear

d= 0.411 m

taking concrete cover to be = 50 mm

D= 461.092 mm

~D= 461.000 mm

hence d= 411.000 mm

B) Wide beam shear

da= 311.44 mm

db= 311.44 mm

d= 411.000 mm

c) Bending moment

Ma-a= 81.52

Mb-b= 81.52

D) Reinforcement

Asmin= 822

Kda-a= 0.0455205 21.96815 Ksa-a= 3.96

Kdb-b= 0.0455205 21.96815 Ksb-b= 3.96

3.97

Asa-a= 822 mm², **Φ12c/c** 12 113

Asb-b= 822 mm², **Φ12c/c** 138

Asb-b= 822 mm², **Φ12c/c** 138

mm
mm

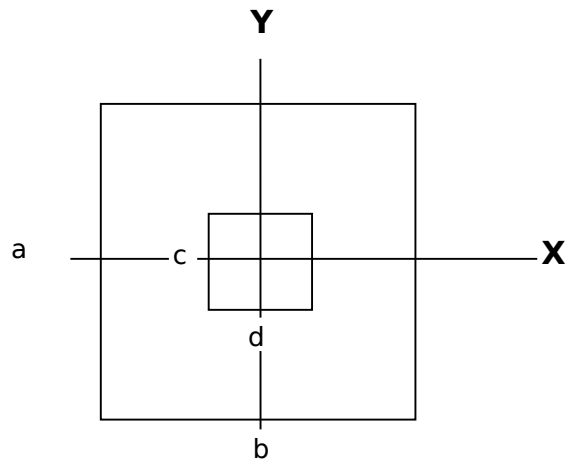
ISOLATED FOOTING DESIGN

I. MAXIMUM DESIGN LOAD

Ps=	442
Soil Wt. & own Wt. Of footing	185.60
Pmax=	627.60 KN
Mx=	1.64 KNm
My=	0.67 KNm

sall= 200 KN/m²

Column dimensions	c=	0.40 m
	d=	0.40 m



S= $P/A(1+6xe_a/a+6e_b/b)$

Where s= the allowable soil pressure

A= axb **ASSUME a/b=** 1.00

a= length of footing 1.78 1.78

b= width of footing 1.78 1.78

P= load sustained by the footing

ea= 0.003

eb= 0.001

s1= 200.00

s2= 199.11

s3= 196.52

s4= 195.10

savg= 197.69

DMT: Use the "tools-Goal seek" command and feed the allowable soil pressure to obtain this result. The value in "E19" should be used as a variable to change in order to achieve this result.

Structural design

use concrete strength 25 Mpa

Fcd= 11.167

Fctd= 1.167

r_{min}= 0.00200

Steel S-300

Vup= 641.67

Vud= 385.00

A) Punching Shear

d= 0.299 m

taking concrete cover to be = 50 mm

D= 349.429 mm

~D= 349.000 mm

hence d= 299.000 mm

B) Wide beam shear

da= 236.30 mm

db= 236.30 mm

d= 299.000 mm

c) Bending moment

Ma-a= 47.22

Mb-b= 47.22

D) Reinforcement

Asmin= 598

Kda-a= 0.0435112 22.98256 Ksa-a= 3.97

Kdb-b= 0.0435112 22.98256 Ksb-b= 3.97 3.97

Asa-a= 626.9871 mm², **Φ12c/c** 12 113 180

Asb-b= 626.9871 mm², **Φ12c/c** 180

mm
mm

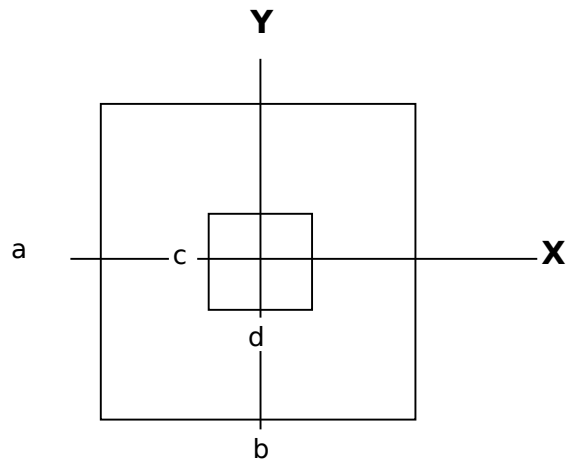
ISOLATED FOOTING DESIGN

I. MAXIMUM DESIGN LOAD

Ps=	1086
Soil Wt. & own Wt. Of footing	449.03
Pmax=	1535.03 KN
Mx=	0.02 KNm
My=	0.3 KNm

sall= 200 KN/m²

Column dimensions	c=	0.40 m
	d=	0.40 m



S= $P/A(1+6xe_a/a+6e_b/b)$

Where s= the allowable soil pressure

A= axb **ASSUME a/b=** 1.00

a= length of footing 2.77 2.77

b= width of footing 2.77 2.77

P= load sustained by the footing

ea= 0.000

eb= 0.000

s1= 200.00

s2= 199.98

s3= 199.99

s4= 199.82

savg= 199.95

DMT: Use the "tools-Goal seek" command and feed the allowable soil pressure to obtain this result. The value in "E19" should be used as a variable to change in order to achieve this result.

Structural design

use concrete strength 25 Mpa

Fcd= 11.167

Fctd= 1.167

r_{min}= 0.00200

Steel S-300

Vup= 641.67

Vud= 385.00

A) Punching Shear

d= 0.553 m

taking concrete cover to be = 50 mm

D= 603.336 mm

~D= 603.000 mm

hence d= 553.000 mm

B) Wide beam shear

da= 405.30 mm

db= 405.30 mm

d= 553.000 mm

c) Bending moment

Ma-a= 140.51

Mb-b= 140.51

D) Reinforcement

Asmin= 1106

Kda-a= 0.0466525 21.43508

Kdb-b= 0.0466525 21.43508

Ksa-a= 3.96

Ksb-b= 3.96
3.97

Asa-a= 1106 mm², **Φ12c/c** 102

Asb-b= 1106 mm², **Φ12c/c** 102

12 113

mm
mm

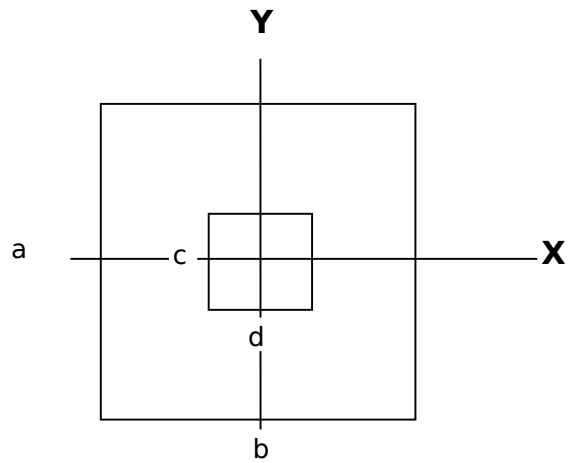
ISOLATED FOOTING DESIGN

I. MAXIMUM DESIGN LOAD

Ps=	1637
Soil Wt. & own Wt. Of footing	501.73
Pmax=	2138.73 KN
Mx=	1.33 KNm
My=	0.57 KNm

sall= 200 KN/m²

Column dimensions	c=	0.40 m
	d=	0.40 m



s= $P/A(1+6xe_a/a+6e_b/b)$

Where s= the allowable soil pressure

A= axb **ASSUME a/b=** 1.25

a= length of footing 3.66 3.66

b= width of footing 2.93 2.93

P= load sustained by the footing

ea= 0.001

eb= 0.000

s1= 200.00

s2= 199.53

s3= 199.59

s4= 199.37

savg= 199.62

DMT: Use the "tools-Goal seek" command and feed the allowable soil pressure to obtain this result. The value in "E19" should be used as a variable to change in order to achieve this result.

Structural design

use concrete strength 25 Mpa

Fcd= 11.167

Fctd= 1.167

r_{min}= 0.00200

Steel S-300

Vup= 641.67

Vud= 385.00

A) Punching Shear

d= 0.685 m

taking concrete cover to be = 50 mm

D= 734.804 mm

~D= 735.000 mm

hence d= 685.000 mm

B) Wide beam shear

da= 557.09 mm

db= 557.09 mm

d= 685.000 mm

c) Bending moment

Ma-a= 265.02

Mb-b= 159.37

D) Reinforcement

Asmin= 1370

Kda-a= 0.0420774 23.76573 Ksa-a= 3.98

Kdb-b= 0.0542617 18.42919 Ksb-b= 3.95 3.97

Asa-a= 1538.552 mm², **Φ16c/c** 14 154 100

Asb-b= 1370 mm², **Φ16c/c** 112

mm
mm